VGZ

UKCA
UKTA-0836
22/6195

ICC ES
AC233
ESR-4645

CE
ETA-11/0030

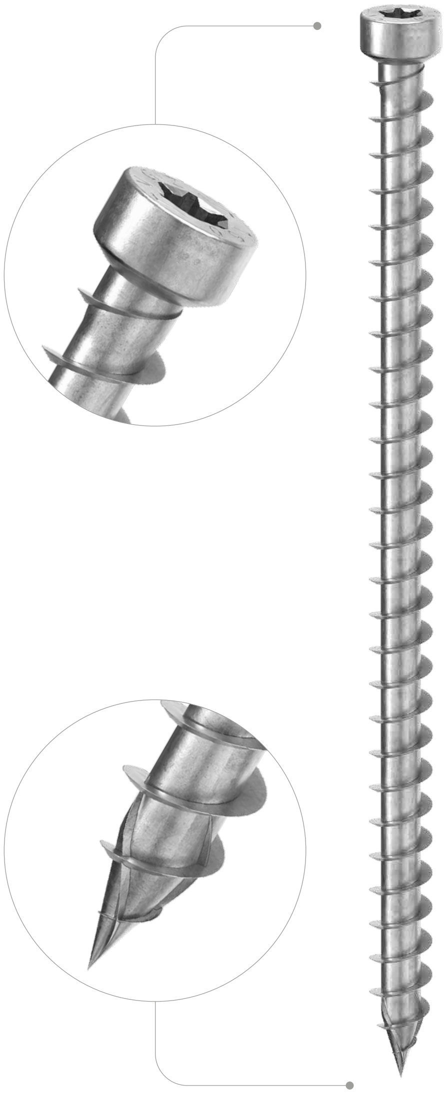
FULL THREADED SCREW WITH CYLINDRICAL HEAD

3 THORNS TIP
Thanks to the 3 THORNS tip, minimum installation distances are reduced. More screws can be used in less space and larger screws in smaller elements. Costs and time for project implementation are reduced.

STRUCTURAL APPLICATIONS
Approved for structural applications subject to stresses in any direction vs the grain (0° ÷ 90°). Cyclical SEISMIC-REV tests according to EN 12512.

CYLINDRICAL HEAD
It allows the screw to penetrate and pass through the surface of the wood substrate. Ideal for concealed joints, timber couplings and structural re-inforcements. It is the right choice to ensure strength in fire conditions.

TIMBER FRAME
Also ideal for joining small timber elements such as the crossbeams and uprights of light frame structures.

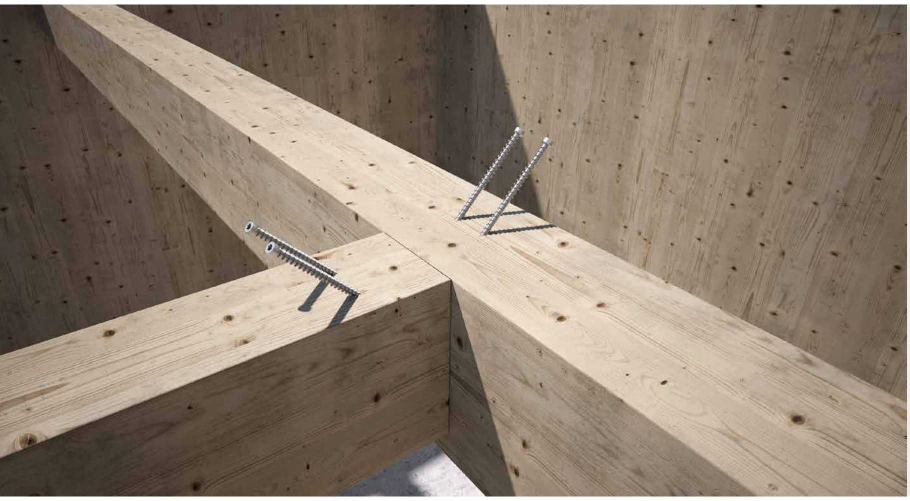


MY PROJECT SOFTWARE

VIDEO

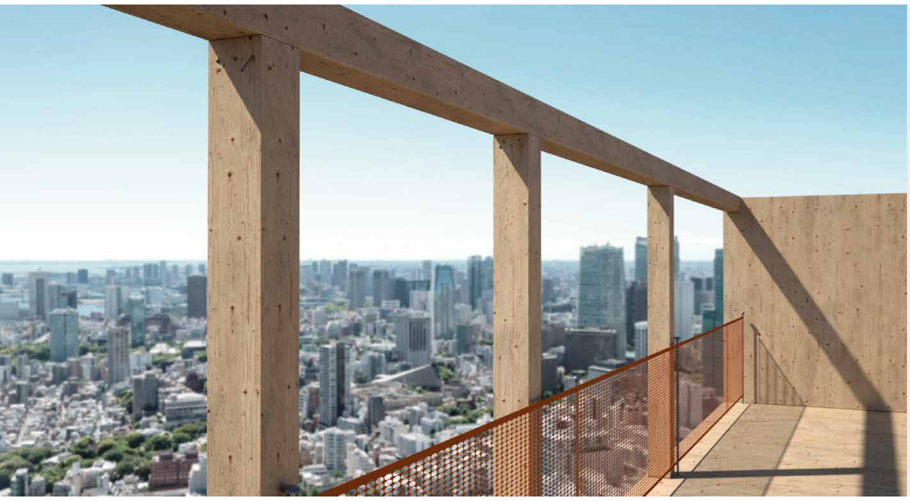
BIT INCLUDED

DIAMETER [mm]	5 (7 – 11) 11
LENGTH [mm]	80 (80 – 1000) 1000
SERVICE CLASS	SC1 SC2
ATMOSPHERIC CORROSIVITY	C1 C2
WOOD CORROSIVITY	T1 T2
MATERIAL	Zn ELECTRO PLATED electrogalvanized carbon steel



FIELDS OF USE

- timber based panels
- solid timber
- glulam (Glued Laminated Timber)
- CLT and LVL
- high density woods

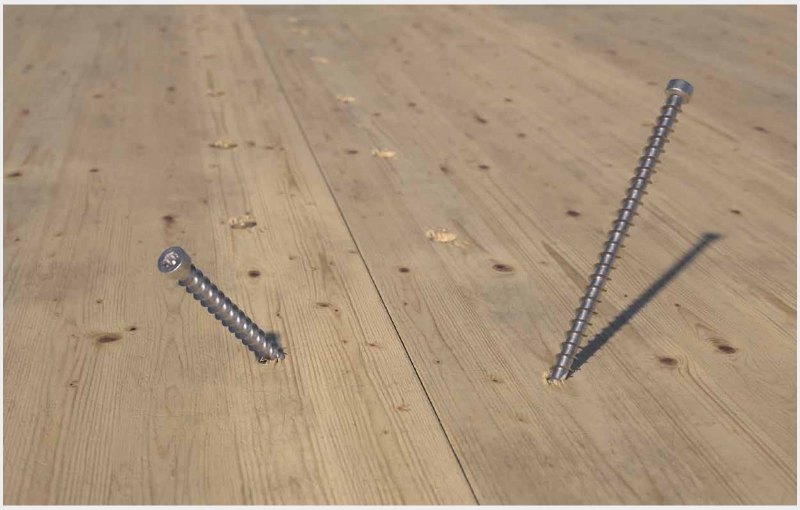


STRUCTURAL RESTORATION

Ideal for coupling beams in structural renovations and new works. Can also be used parallel to the grain thanks to the special approval.

CLT, LVL

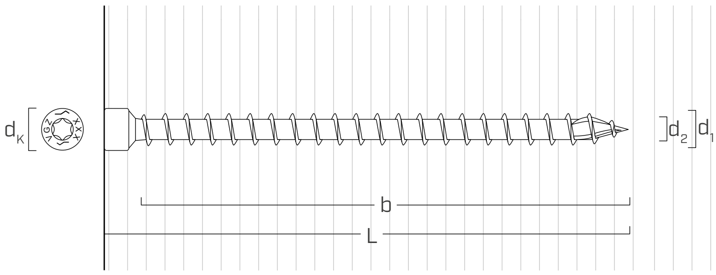
Values also tested, certified and calculated for CLT and high density woods such as Microllam® LVL.

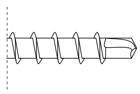


Very high stiffness in side-by-side joining of CLT floors. Application with double inclination at 45°, perfect combined with the JIG VGZ template.

Reinforcement orthogonal to grain for hanging load due to joining of main-secondary beams.

GEOMETRY AND MECHANICAL CHARACTERISTICS





Ø9 | L > 520 mm
Ø11 | L > 600 mm

GEOMETRY

Nominal diameter	d_1	[mm]	7	9	11
Head diameter	d_K	[mm]	9,50	11,50	13,50
Thread diameter	d_2	[mm]	4,60	5,90	6,60
Pre-drilling hole diameter(1)	$d_{V,S}$	[mm]	4,0	5,0	6,0
Pre-drilling hole diameter(2)	$d_{V,H}$	[mm]	5,0	6,0	7,0

(1) Pre-drilling valid for softwood.
(2) Pre-drilling valid for hardwood and beech LVL.

CHARACTERISTIC MECHANICAL PARAMETERS

Nominal diameter	d_1	[mm]	7	9	11
Tensile strength	$f_{tens,k}$	[kN]	15,4	25,4	38,0
Yield strength	$f_{y,k}$	[N/mm^2]	1000	1000	1000
Yield moment	$M_{y,k}$	[Nm]	14,2	27,2	45,9

			softwood *(softwood)*	**LVL softwood** *(LVL softwood)*	**pre-drilled beech LVL** *(beech LVL predrilled)*
Withdrawal resistance parameter	$f_{ax,k}$	[N/mm^2]	11,7	15,0	29,0
Associated density	ρ_a	[kg/m^3]	350	500	730
Calculation density	ρ_k	*[kg/m^3]*	*≤ 440*	*410 ÷ 550*	*590 ÷ 750*

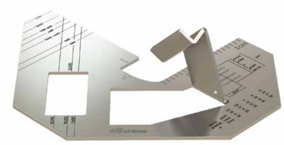
For applications with different materials please see ETA-11/0030.

CODES AND DIMENSIONS

d_1 [mm]	CODE	L [mm]	b [mm]	pcs
7 TX 30	VGZ780	80	70	25
	VGZ7100	100	90	25
	VGZ7120	120	110	25
	VGZ7140	140	130	25
	VGZ7160	160	150	25
	VGZ7180	180	170	25
	VGZ7200	200	190	25
	VGZ7220	220	210	25
	VGZ7240	240	230	25
	VGZ7260	260	250	25
	VGZ7280	280	270	25
	VGZ7300	300	290	25
	VGZ7320	320	310	25
	VGZ7340	340	330	25
	VGZ7360	360	350	25
	VGZ7380	380	370	25
	VGZ7400	400	390	25
9 TX 40	VGZ9160	160	150	25
	VGZ9180	180	170	25
	VGZ9200	200	190	25
	VGZ9220	220	210	25
	VGZ9240	240	230	25
	VGZ9260	260	250	25
	VGZ9280	280	270	25
	VGZ9300	300	290	25
	VGZ9320	320	310	25
	VGZ9340	340	330	25
	VGZ9360	360	350	25
	VGZ9380	380	370	25
	VGZ9400	400	390	25
	VGZ9440	440	430	25
	VGZ9480	480	470	25
	VGZ9520	520	510	25
	VGZ9560	560	550	25
	VGZ9600	600	590	25

d_1 [mm]	CODE	L [mm]	b [mm]	pcs
11 TX 50	VGZ11150	150	140	25
	VGZ11200	200	190	25
	VGZ11250	250	240	25
	VGZ11275	275	265	25
	VGZ11300	300	290	25
	VGZ11325	325	315	25
	VGZ11350	350	340	25
	VGZ11375	375	365	25
	VGZ11400	400	390	25
	VGZ11425	425	415	25
	VGZ11450	450	440	25
	VGZ11475	475	465	25
	VGZ11500	500	490	25
	VGZ11525	525	515	25
	VGZ11550	550	540	25
	VGZ11575	575	565	25
	VGZ11600	600	590	25
	VGZ11650	650	640	25
	VGZ11700	700	690	25
	VGZ11750	750	740	25
	VGZ11800	800	790	25
	VGZ11850	850	840	25
	VGZ11900	900	890	25
	VGZ11950	950	940	25
	VGZ111000	1000	990	25

RELATED PRODUCTS



JIG VGZ 45°

TEMPLATE FOR 45° SCREWS

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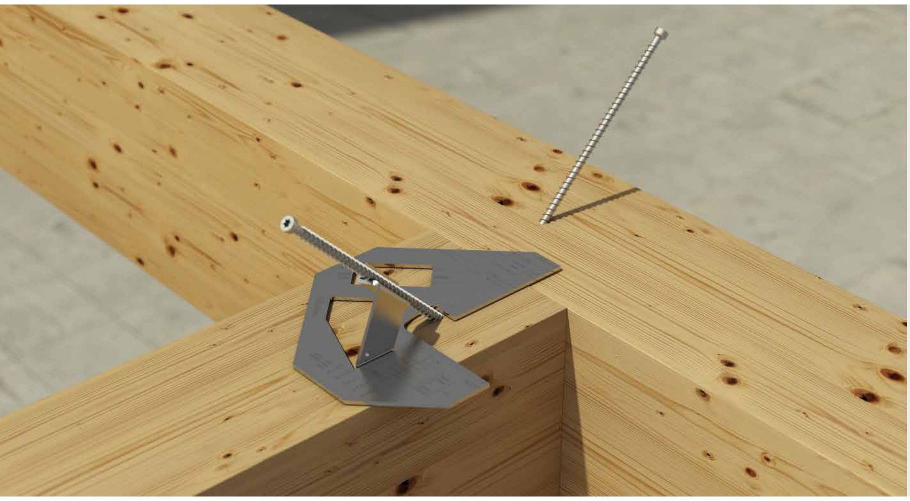
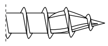


JIG VGZ 45° TEMPLATE

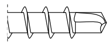
Installation at 45° using the JIG VGZ steel template.

MINIMUM DISTANCES FOR AXIAL STRESSES | TIMBER

screws inserted **WITH and WITHOUT pre-drilled hole**

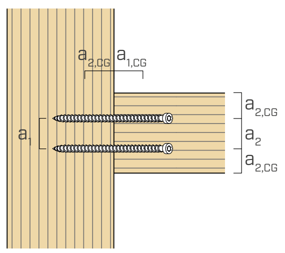


d_1	[mm]		7	9	11
a_1	[mm]	5·d	35	45	55
a_2	[mm]	5·d	35	45	55
$a_{2,LIM}$	[mm]	2,5·d	18	23	28
$a_{1,CG}$	[mm]	8·d	56	72	88
$a_{2,CG}$	[mm]	3·d	21	27	33
a_{CROSS}	[mm]	1,5·d	11	14	17

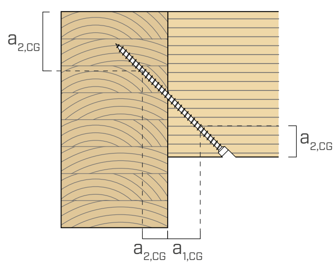


d_1	[mm]		9	11
a_1	[mm]	5·d	45	55
a_2	[mm]	5·d	45	55
$a_{2,LIM}$	[mm]	2,5·d	23	28
$a_{1,CG}$	[mm]	5·d	45	55
$a_{2,CG}$	[mm]	3·d	27	33
a_{CROSS}	[mm]	1,5·d	14	17

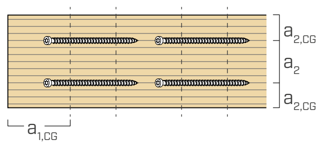
SCREWS UNDER TENSION INSERTED WITH AN ANGLE α WITH RESPECT TO THE GRAIN



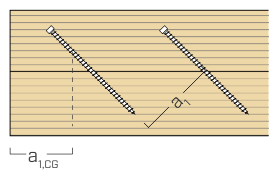
plan



front

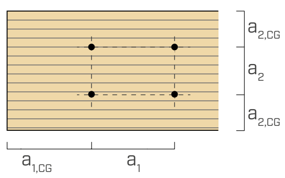


plan

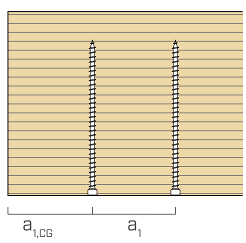


front

SCREWS INSERTED WITH α = 90° ANGLE WITH RESPECT TO THE GRAIN

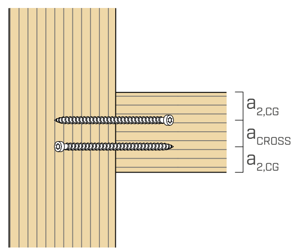


plan

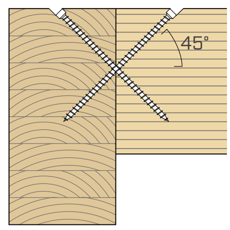


front

CROSSED SCREWS INSERTED WITH AN ANGLE α WITH RESPECT TO THE GRAIN



plan

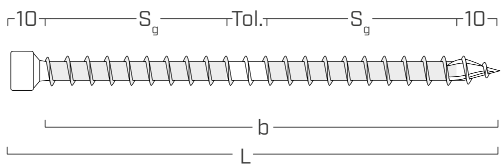


front

NOTES

- Minimum distances according to ETA-11/0030.
- The minimum distances are independent of the insertion angle of the connector and the angle of the force with respect to the grain.
- The axial distance a_2 can be reduced down to $a_{2,LIM}$ if for each connector a "joint surface" $a_1\ a_2 = 25\ d_1^2$ is maintained.
- For main beam-secondary beam joints with VGZ screws d = 7 mm inclined or crossed, inserted at an angle of 45° to the secondary beam head, with a minimum secondary beam height of 18 d, the minimum distance $a_{1,CG}$ can be taken equal to $8 \cdot d_1$ anc the minimum distance $a_{2,CG}$ equal to $3 \cdot d_1$.
- For 3 THORNS tip and self-drilling tip screws, the minimum distances in the table are derived from experimental tests; alternatively, adopt $a_{1,CG} = 10 \cdot d$ and $a_{2,CG} = 4 \cdot d$ in accordance with EN 1995:2014.

EFFECTIVE THREAD USED IN CALCULATION

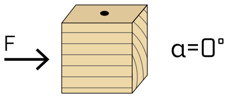


$b = S_{g,tot} = L - 10\ mm$ represents the entire length of the threaded part

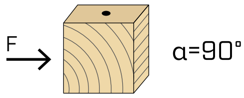
$S_g = (L - 10\ mm - 10\ mm - Tol.)/\ 2$ represents the partial length of the threaded part net of a laying tolerance (Tol.) of 10 mm

MINIMUM DISTANCES FOR SHEAR LOADS | TIMBER

screws inserted **WITHOUT pre-drilled hole** $\rho_k \leq$ **420 kg/m^3**



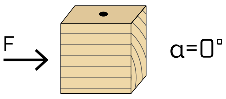
d_1	[mm]		7	9	11
a_1	[mm]	**10·d**	70	90	110
a_2	[mm]	**5·d**	35	45	55
$a_{3,t}$	[mm]	**15·d**	105	135	165
$a_{3,c}$	[mm]	**10·d**	70	90	110
$a_{4,t}$	[mm]	**5·d**	35	45	55
$a_{4,c}$	[mm]	**5·d**	35	45	55



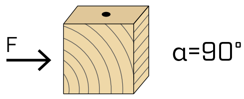
d_1	[mm]		7	9	11
a_1	[mm]	**5·d**	35	45	55
a_2	[mm]	**5·d**	35	45	55
$a_{3,t}$	[mm]	**10·d**	70	90	110
$a_{3,c}$	[mm]	**10·d**	70	90	110
$a_{4,t}$	[mm]	**10·d**	70	90	110
$a_{4,c}$	[mm]	**5·d**	35	45	55

α = load-to-grain angle
d = d_1 = nominal screw diameter

screws inserted **WITH pre-drilled hole**



d_1	[mm]		7	9	11
a_1	[mm]	**5·d**	35	45	55
a_2	[mm]	**3·d**	21	27	33
$a_{3,t}$	[mm]	**12·d**	84	108	132
$a_{3,c}$	[mm]	**7·d**	49	63	77
$a_{4,t}$	[mm]	**3·d**	21	27	33
$a_{4,c}$	[mm]	**3·d**	21	27	33



d_1	[mm]		7	9	11
a_1	[mm]	**4·d**	28	36	44
a_2	[mm]	**4·d**	28	36	44
$a_{3,t}$	[mm]	**7·d**	49	63	77
$a_{3,c}$	[mm]	**7·d**	49	63	77
$a_{4,t}$	[mm]	**7·d**	49	63	77
$a_{4,c}$	[mm]	**3·d**	21	27	33

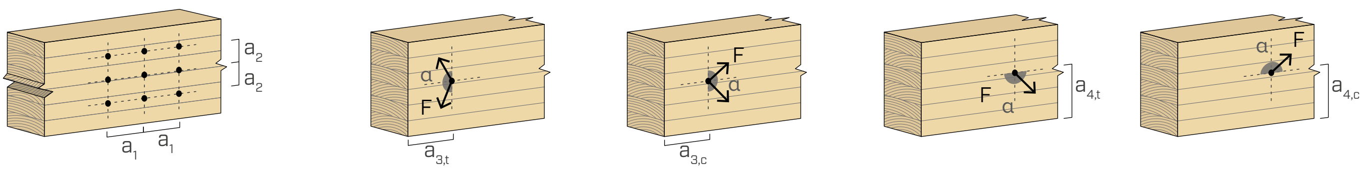
α = load-to-grain angle
d = d_1 = nominal screw diameter

stressed end
-90° < α < 90°

unloaded end
90° < α < 270°

stressed edge
0° < α < 180°

unload edge
180° < α < 360°



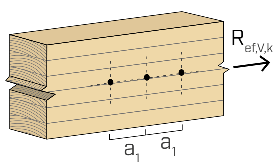
NOTES

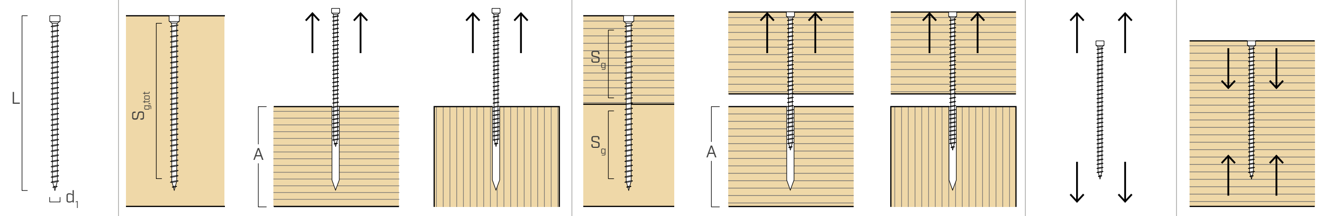
- The minimum distances comply with the EN 1995:2014 standard in accordance with ETA-11/0030.
- The minimum spacing for all panel-to-timber connections (a_1, a_2) can be multiplied by a coefficient of 0,85.
- The spacing a_1 in the table for screws with 3 THORNS tip inserted without pre-drilling hole in timber elements with density $\rho_k \leq 420$ kg/m^3 and load-to-grain angle α = 0° was assumed to be 10·d based on experimental tests; alternatively, adopt 12·d in accordance with EN 1995:2014.

EFFECTIVE NUMBER FOR SHEAR LOADS

The load-bearing capacity of a connection made with several screws, all of the same type and size, may be lower than the sum of the load-bearing capacities of the individual connection system.

For a row of n screws arranged parallel to the direction of the grain at a distance a_1, the characteristic effective shear bearing capacity $R_{ef,V,k}$ can be calculated by means of the effective number n_{ef} (see page 169).

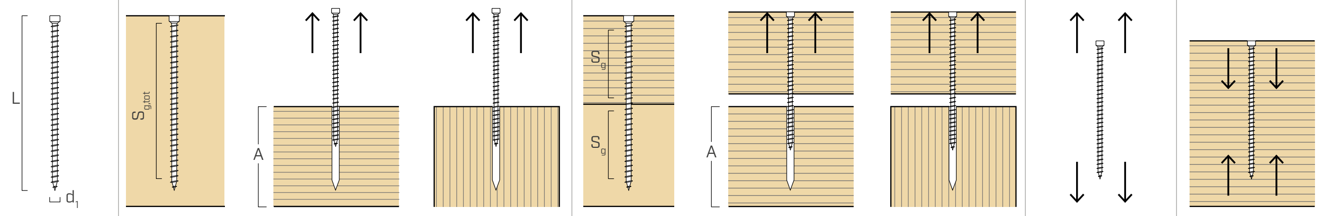




		TENSION / COMPRESSION									
geometry		total thread withdrawal				partial thread withdrawal				steel tension	instability
			ε=90°		ε=0°		ε=90°		ε=0°		ε=90°
d_1 [mm]	L [mm]	$S_{g,tot}$ [mm]	A_{min} [mm]	$R_{ax,90,k}$ [kN]	$R_{ax,0,k}$ [kN]	S_g [mm]	A_{min} [mm]	$R_{ax,90,k}$ [kN]	$R_{ax,0,k}$ [kN]	$R_{tens,k}$ [kN]	$R_{ki,90,k}$ [kN]
	80	70	90	6,19	1,86	-	-	-	-		
	100	90	110	7,96	2,39	35	55	3,09	0,93		
	120	110	130	9,72	2,92	45	65	3,98	1,19		
	140	130	150	11,49	3,45	55	75	4,86	1,46		
	160	150	170	13,26	3,98	65	85	5,75	1,72		
	180	170	190	15,03	4,51	75	95	6,63	1,99		
	200	190	210	16,79	5,04	85	105	7,51	2,25		
	220	210	230	18,56	5,57	95	115	8,40	2,52		
7	240	230	250	20,33	6,10	105	125	9,28	2,78	15,40	10,30
	260	250	270	22,10	6,63	115	135	10,16	3,05		
	280	270	290	23,87	7,16	125	145	11,05	3,31		
	300	290	310	25,63	7,69	135	155	11,93	3,58		
	320	310	330	27,40	8,22	145	165	12,82	3,84		
	340	330	350	29,17	8,75	155	175	13,70	4,11		
	360	350	370	30,94	9,28	165	185	14,58	4,38		
	380	370	390	32,70	9,81	175	195	15,47	4,64		
	400	390	410	34,47	10,34	185	205	16,35	4,91		
	160	150	170	17,05	5,11	65	85	7,39	2,22		
	180	170	190	19,32	5,80	75	95	8,52	2,56		
	200	190	210	21,59	6,48	85	105	9,66	2,90		
	220	210	230	23,87	7,16	95	115	10,80	3,24		
	240	230	250	26,14	7,84	105	125	11,93	3,58		
	260	250	270	28,41	8,52	115	135	13,07	3,92		
	280	270	290	30,68	9,21	125	145	14,21	4,26		
	300	290	310	32,96	9,89	135	155	15,34	4,60		
9	320	310	330	35,23	10,57	145	165	16,48	4,94	25,40	17,25
	340	330	350	37,50	11,25	155	175	17,61	5,28		
	360	350	370	39,78	11,93	165	185	18,75	5,63		
	380	370	390	42,05	12,61	175	195	19,89	5,97		
	400	390	410	44,32	13,30	185	205	21,02	6,31		
	440	430	450	48,87	14,66	205	225	23,30	6,99		
	480	470	490	53,41	16,02	225	245	25,57	7,67		
	520	510	530	57,96	17,39	245	265	27,84	8,35		
	560	550	570	62,50	18,75	265	285	30,12	9,03		
	600	590	610	67,05	20,11	285	305	32,39	9,72		

ε = screw-to-grain angle

TENSION / COMPRESSION



geometry		total thread withdrawal ε=90°			total thread withdrawal ε=0°	partial thread withdrawal ε=90°			partial thread withdrawal ε=0°	steel tension	instability ε=90°
d_1 [mm]	L [mm]	$S_{g,tot}$ [mm]	A_{min} [mm]	$R_{ax,90,k}$ [kN]	$R_{ax,0,k}$ [kN]	S_g [mm]	A_{min} [mm]	$R_{ax,90,k}$ [kN]	$R_{ax,0,k}$ [kN]	$R_{tens,k}$ [kN]	$R_{ki,90,k}$ [kN]
	150	140	160	19,45	5,83	60	80	8,33	2,50		
	200	190	210	26,39	7,92	85	105	11,81	3,54		
	250	240	260	33,34	10,00	110	130	15,28	4,58		
	275	265	285	36,81	11,04	123	143	17,01	5,10		
	300	290	310	40,28	12,08	135	155	18,75	5,63		
	325	315	335	43,75	13,13	148	168	20,49	6,15		
	350	340	360	47,22	14,17	160	180	22,22	6,67		
	375	365	385	50,70	15,21	173	193	23,96	7,19		
	400	390	410	54,17	16,25	185	205	25,70	7,71		
	425	415	435	57,64	17,29	198	218	27,43	8,23		
	450	440	460	61,11	18,33	210	230	29,17	8,75		
	475	465	485	64,59	19,38	223	243	30,90	9,27		
11	500	490	510	68,06	20,42	235	255	32,64	9,79	38,00	21,93
	525	515	535	71,53	21,46	248	268	34,38	10,31		
	550	540	560	75,00	22,50	260	280	36,11	10,83		
	575	565	585	78,48	23,54	273	293	37,85	11,35		
	600	590	610	81,95	24,58	285	305	39,59	11,88		
	650	640	660	88,89	26,67	310	330	43,06	12,92		
	700	690	710	95,84	28,75	335	355	46,53	13,96		
	750	740	760	102,78	30,84	360	380	50,00	15,00		
	800	790	810	109,73	32,92	385	405	53,48	16,04		
	850	840	860	116,67	35,00	410	430	56,95	17,08		
	900	890	910	123,62	37,09	435	455	60,42	18,13		
	950	940	960	130,56	39,17	460	480	63,89	19,17		
	1000	990	1010	137,51	41,25	485	505	67,37	20,21		

ε = screw-to-grain angle

NOTES

- The characteristic thread withdrawal resistances were evaluated considering both an ε angle of 90° ($R_{ax,90,k}$) and of 0° ($R_{ax,0,k}$) between the grains of the timber element and the connector.
- For the calculation process a timber characteristic density ρ_k = 385 kg/m^3 has been considered.
  For different ρ_k values, the strength values in the table can be converted by the k_{dens} coefficient.

$$R'_{ax,k} = k_{dens,ax} \cdot R_{ax,k}$$
$$R'_{ki,k} = k_{dens,ki} \cdot R_{ki,k}$$

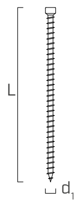
ρ_k [kg/m^3]	350	380	**385**	405	425	430	440
C-GL	*C24*	*C30*	*GL24h*	*GL26h*	*GL28h*	*GL30h*	*GL32h*
$k_{dens,ax}$	0,92	0,98	1,00	1,04	1,08	1,09	1,11
$k_{dens,ki}$	0,97	0,99	1,00	1,00	1,01	1,02	1,02

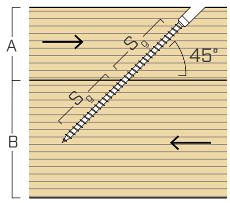
Strength values thus determined may differ, for higher safety standards, from those resulting from an exact calculation.

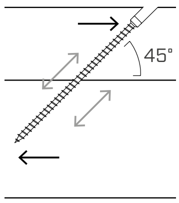
GENERAL PRINCIPLES on page 143.

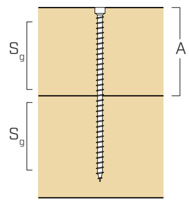
STRUCTURAL VALUES | TIMBER

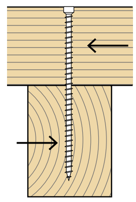
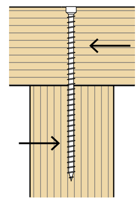
CHARACTERISTIC VALUES
EN 1995:2014





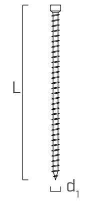
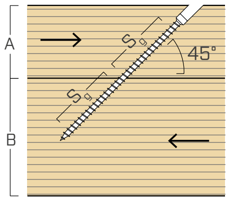
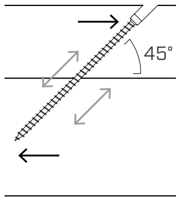
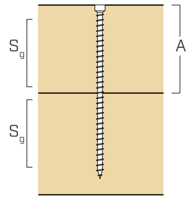
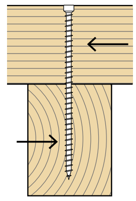
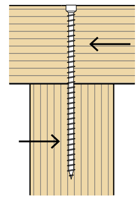




		SLIDING					SHEAR			
geometry		timber-to-timber				steel tension	timber-to-timber		timber-to-timber ε=90°	timber-to-timber ε=0°
d_1 [mm]	L [mm]	S_g [mm]	A [mm]	B_{min} [mm]	$R_{V,k}$ [kN]	$R_{tens,45,k}$ [kN]	A [mm]	S_g [mm]	$R_{V,90,k}$ [mm]	$R_{V,0,k}$ [kN]
7	80	-	-	-	-	10,89	40	25	2,59	1,34
	100	35	40	55	2,19		50	35	2,93	1,53
	120	45	45	60	2,81		60	45	3,15	1,74
	140	55	55	70	3,44		70	55	3,37	1,97
	160	65	60	75	4,06		80	65	3,59	2,06
	180	75	70	85	4,69		90	75	3,81	2,12
	200	85	75	90	5,31		100	85	4,03	2,19
	220	95	85	100	5,94		110	95	4,25	2,26
	240	105	90	105	6,56		120	105	4,30	2,32
	260	115	95	110	7,19		130	115	4,30	2,39
	280	125	105	120	7,81		140	125	4,30	2,46
	300	135	110	125	8,44		150	135	4,30	2,52
	320	145	120	135	9,06		160	145	4,30	2,59
	340	155	125	140	9,69		170	155	4,30	2,65
	360	165	130	145	10,31		180	165	4,30	2,72
	380	175	140	155	10,94		190	175	4,30	2,79
	400	185	145	160	11,56		200	185	4,30	2,85
9	160	65	60	75	5,22	17,96	80	65	5,10	2,81
	180	75	70	85	6,03		90	75	5,38	3,08
	200	85	75	90	6,83		100	85	5,67	3,18
	220	95	85	100	7,63		110	95	5,95	3,27
	240	105	90	105	8,44		120	105	6,23	3,35
	260	115	95	110	9,24		130	115	6,50	3,44
	280	125	105	120	10,04		140	125	6,50	3,52
	300	135	110	125	10,85		150	135	6,50	3,61
	320	145	120	135	11,65		160	145	6,50	3,69
	340	155	125	140	12,46		170	155	6,50	3,78
	360	165	130	145	13,26		180	165	6,50	3,86
	380	175	140	155	14,06		190	175	6,50	3,95
	400	185	145	160	14,87		200	185	6,50	4,03
	440	205	160	175	16,47		220	205	6,50	4,21
	480	225	175	190	18,08		240	225	6,50	4,38
	520	245	190	205	19,69		260	245	6,50	4,55
	560	265	205	220	21,29		280	265	6,50	4,72
	600	285	215	230	22,90		300	285	6,50	4,89

ε = screw-to-grain angle

		SLIDING					SHEAR			
geometry		timber-to-timber				steel tension	timber-to-timber		timber-to-timber ε=90°	timber-to-timber ε=0°
										
d_1 [mm]	L [mm]	S_g [mm]	A [mm]	B_{min} [mm]	$R_{V,k}$ [kN]	$R_{tens,45,k}$ [kN]	A [mm]	S_g [mm]	$R_{V,90,k}$ [mm]	$R_{V,0,k}$ [kN]
	150	60	60	75	5,89		75	60	6,61	3,33
	200	85	75	90	8,35		100	85	7,48	4,10
	250	110	95	110	10,80		125	110	8,35	4,57
	275	123	100	115	12,03		138	123	8,79	4,70
	300	135	110	125	13,26		150	135	9,06	4,83
	325	148	120	135	14,49		163	148	9,06	4,96
	350	160	130	145	15,71		175	160	9,06	5,09
	375	173	140	155	16,94		188	173	9,06	5,22
	400	185	145	160	18,17		200	185	9,06	5,35
	425	198	155	170	19,40		213	198	9,06	5,48
	450	210	165	180	20,63		225	210	9,06	5,61
	475	223	175	190	21,85		238	223	9,06	5,74
11	500	235	180	195	23,08	26,87	250	235	9,06	5,87
	525	248	190	205	24,31		263	248	9,06	6,00
	550	260	200	215	25,54		275	260	9,06	6,13
	575	273	210	225	26,76		288	273	9,06	6,26
	600	285	215	230	27,99		300	285	9,06	6,39
	650	310	235	250	30,45		325	310	9,06	6,65
	700	335	250	265	32,90		350	335	9,06	6,85
	750	360	270	285	35,36		375	360	9,06	6,85
	800	385	290	305	37,81		400	385	9,06	6,85
	850	410	305	320	40,27		425	410	9,06	6,85
	900	435	325	340	42,72		450	435	9,06	6,85
	950	460	340	355	45,18		475	460	9,06	6,85
	1000	485	360	375	47,63		500	485	9,06	6,85

ε = screw-to-grain angle

NOTES

- The characteristic sliding strengths were evaluated by considering an angle ε of 45° between the grains of the timber element and the connector.
- The characteristic timber-to-timber shear strengths were evaluated considering both an ε angle of 90° ($R_{V,90,k}$) and 0° ($R_{V,0,k}$) between the grains of the second element and the connector.
- For the calculation process a timber characteristic density ρ_k = 385 kg/m^3 has been considered.
  For different ρ_k values, the strength values in the table can be converted by the k_{dens} coefficient.

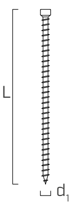
$$R'_{V,k} = k_{dens,ax} \cdot R_{V,k}$$
$$R'_{V,90,k} = k_{dens,V} \cdot R_{V,90,k}$$
$$R'_{V,0,k} = k_{dens,V} \cdot R_{V,0,k}$$

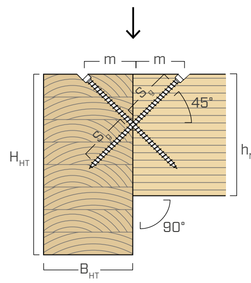
ρ_k [kg/m^3]	350	380	**385**	405	425	430	440
C-GL	*C24*	*C30*	*GL24h*	*GL26h*	*GL28h*	*GL30h*	*GL32h*
$k_{dens,ax}$	0,92	0,98	1,00	1,04	1,08	1,09	1,11
$k_{dens,v}$	0,90	0,98	1,00	1,02	1,05	1,05	1,07

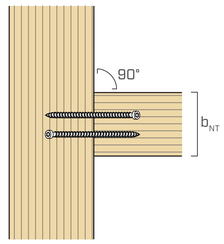
Strength values thus determined may differ, for higher safety standards, from those resulting from an exact calculation.

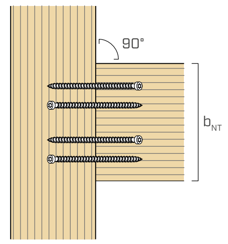
GENERAL PRINCIPLES on page 143.

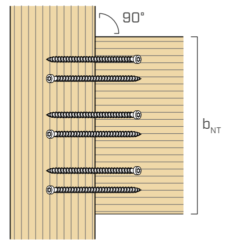
MAIN BEAM-SECONDARY BEAM SHEAR CONNECTION







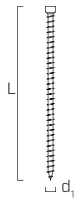


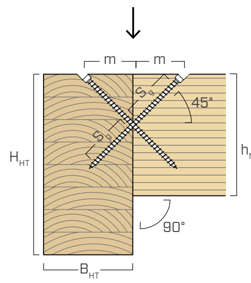


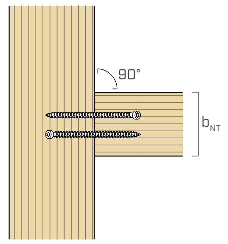
geometry		main beam secondary beam				1 pair			2 pairs			3 pairs		
d_1	L	$B_{HT,min}$	$H_{HT,min}$ $h_{NT,min}$	S_g	m	$b_{NT,min}$	$R_{V1,k}$	$R_{V2,k}$	$b_{NT,min}$	$R_{V1,k}$	$R_{V2,k}$	$b_{NT,min}$	$R_{V1,k}$	$R_{V2,k}$
[mm]	[mm]	[mm]	[mm]	[mm]	[mm]	[mm]	[kN]	[kN]	[mm]	[kN]	[kN]	[mm]	[kN]	[kN]
	160	75	130	65	60	53	8,13		88	15,16		123	21,84	
	180	80	140	75	67	53	9,38		88	17,49		123	25,20	
	200	90	155	85	74	53	10,63		88	19,83		123	28,56	
	220	95	170	95	81	53	11,88		88	22,16		123	31,92	
	240	100	185	105	88	53	13,13		88	24,49		123	35,28	
	260	110	200	115	95	53	14,38		88	26,82		123	38,64	
7	280	115	210	125	102	53	15,63	13,63	88	29,16	25,44	123	42,00	36,64
	300	125	225	135	109	53	16,88		88	31,49		123	45,36	
	320	130	240	145	116	53	18,13		88	33,82		123	48,72	
	340	140	255	155	123	53	19,38		88	36,16		123	52,08	
	360	145	270	165	130	53	20,63		88	38,49		123	55,44	
	380	150	285	175	137	53	21,78		88	40,64		123	58,54	
	400	160	295	185	144	53	21,78		88	40,64		123	58,54	
	200	90	155	85	74	68	13,66		113	25,49		158	36,72	
	220	95	170	95	81	68	15,27		113	28,49		158	41,04	
	240	100	185	105	88	68	16,88		113	31,49		158	45,36	
	260	110	200	115	95	68	18,48		113	34,49		158	49,68	
	280	115	210	125	102	68	20,09		113	37,49		158	54,00	
	300	125	225	135	109	68	21,70		113	40,49		158	58,32	
	320	130	240	145	116	68	23,30		113	43,49		158	62,64	
9	340	140	255	155	123	68	24,91	22,88	113	46,49	42,69	158	66,96	61,50
	360	145	270	165	130	68	26,52		113	49,48		158	71,28	
	380	150	285	175	137	68	28,13		113	52,48		158	75,60	
	400	160	295	185	144	68	29,73		113	55,48		158	79,92	
	440	175	325	205	159	68	32,95		113	61,48		158	88,56	
	480	185	355	225	173	68	35,92		113	67,03		158	96,55	
	520	200	380	245	187	68	35,92		113	67,03		158	96,55	
	560	215	410	265	201	68	35,92		113	67,03		158	96,55	
	600	230	440	285	215	68	35,92		113	67,03		158	96,55	

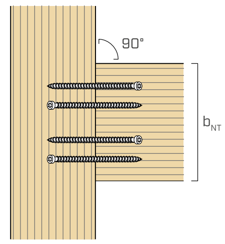
MAIN BEAM-SECONDARY BEAM SHEAR CONNECTION

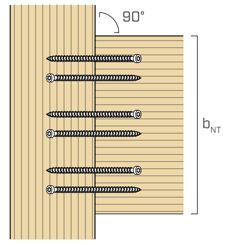
geometry		main beam secondary beam				1 pair			2 pairs			3 pairs		











d_1 [mm]	L [mm]	$B_{HT,min}$ [mm]	$H_{HT,min}$ $h_{NT,min}$ [mm]	S_g [mm]	m [mm]	$b_{NT,min}$ [mm]	$R_{V1,k}$ [kN]	$R_{V2,k}$ [kN]	$b_{NT,min}$ [mm]	$R_{V1,k}$ [kN]	$R_{V2,k}$ [kN]	$b_{NT,min}$ [mm]	$R_{V1,k}$ [kN]	$R_{V2,k}$ [kN]
	250	105	190	110	91	83	21,61		138	40,32		193	58,08	
	275	115	210	125	102	83	24,55		138	45,82		193	66,00	
	300	125	225	135	109	83	26,52		138	49,48		193	71,28	
	325	135	250	150	120	83	29,46		138	54,98		193	79,20	
	350	140	260	160	127	83	31,43		138	58,65		193	84,48	
	375	150	285	175	137	83	34,38		138	64,15		193	92,40	
	400	160	295	185	144	83	36,34		138	67,81		193	97,68	
	425	170	320	200	155	83	39,29		138	73,31		193	105,60	
	450	175	335	210	162	83	41,25		138	76,98		193	110,88	
	475	185	355	225	173	83	44,20		138	82,47		193	118,80	
	500	195	370	235	180	83	46,16		138	86,14		193	124,08	
11	525	205	390	250	190	83	49,11	29,15	138	91,64	54,40	193	131,99	78,35
	550	210	405	260	197	83	51,07		138	95,30		193	137,27	
	575	225	425	275	208	83	53,74		138	100,28		193	144,45	
	600	230	440	285	215	83	53,74		138	100,28		193	144,45	
	650	245	475	310	233	83	53,74		138	100,28		193	144,45	
	700	265	510	335	251	83	53,74		138	100,28		193	144,45	
	750	285	545	360	268	83	53,74		138	100,28		193	144,45	
	800	300	580	385	286	83	53,74		138	100,28		193	144,45	
	850	320	615	410	304	83	53,74		138	100,28		193	144,45	
	900	335	650	435	321	83	53,74		138	100,28		193	144,45	
	950	355	685	460	339	83	53,74		138	100,28		193	144,45	
	1000	370	720	485	357	83	53,74		138	100,28		193	144,45	

NOTES

- The compression design strength of the connectors is the lower between the withdrawal-side design strength ($R_{V1,d}$) and the instability design strength ($R_{V2,d}$).

$$R_{V,d} = \min \begin{cases} \dfrac{R_{V1,k} \cdot k_{mod}}{\gamma_M} \\ \dfrac{R_{V2,k}}{\gamma_{M1}} \end{cases}$$

- The values given are calculated considering a distance $a_{1,CG} \geq 5d$.
- For the calculation process a timber characteristic density ρ_k = 385 kg/m^3 has been considered.
For different ρ_k values, the strength values in the table can be converted by the k_{dens} coefficients previously indicated:

$$R'_{V1,k} = k_{dens,ax} \cdot R_{V1,k}$$
$$R'_{V2,k} = k_{dens,ki} \cdot R_{V2,k}$$

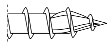
Strength values thus determined may differ, for higher safety standards, from those resulting from an exact calculation.

- The assembly figure (m) is valid in the case of symmetrical installation of the connectors flush over the elements.
- The connectors must be inserted at 45° with respect to the shear plane.
- The strength values in the table for connections with several pairs of crossed screws are already inclusive of $n_{ef,ax}$.

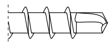
GENERAL PRINCIPLES on page 143.

MINIMUM DISTANCES FOR CROSSED CONNECTORS

screws inserted **WITH and WITHOUT pre-drilled hole**

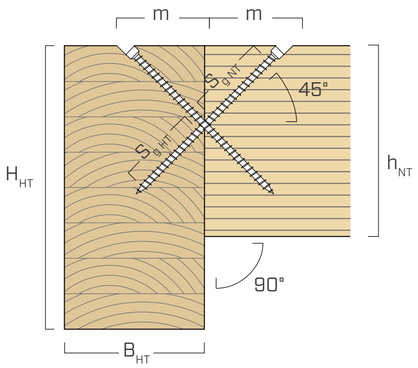


d_1	[mm]		7	9	11
$a_{2,CG}$	[mm]	3·d	21	27	33
a_{CROSS}	[mm]	1,5·d	11	14	17
e	[mm]	3,5·d	25	32	39

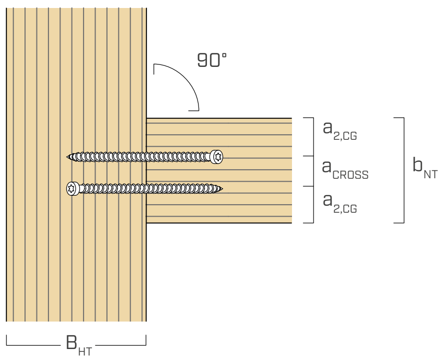


d_1	[mm]		9	11
$a_{2,CG}$	[mm]	3·d	27	33
a_{CROSS}	[mm]	1,5·d	14	17
e	[mm]	3,5·d	32	39

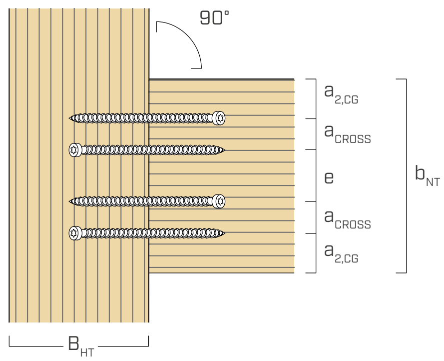
$d = d_1$ = nominal screw diameter



section



plan - 1 PAIR



plan - 2 OR MORE PAIRS

NOTES

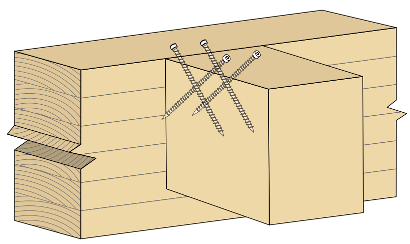
- For main beam-secondary beam joints with VGZ screws d = 7 mm inclined or crossed, inserted at an angle of 45° to the secondary beam head, with a minimum secondary beam height of 18 d, the minimum distance $a_{1,CG}$ can be taken equal to $8 \cdot d_1$ anc the minimum distance $a_{2,CG}$ equal to $3 \cdot d_1$.
- For 3 THORNS tip and self-drilling tip screws, the minimum distances in the table are derived from experimental tests; alternatively, adopt $a_{1,CG} = 10 \cdot d$ and $a_{2,CG} = 4 \cdot d$ in accordance with EN1995:2014.

EFFECTIVE NUMBER FOR AXIALLY STRESSED CONNECTOR PAIRS

The load-bearing capacity of a connection made with several screws, all of the same type and size, may be lower than the sum of the load-bearing capacities of the individual connection system.

For a connection with n pairs of crossed screws, the characteristic effective load-bearing capacity is equal to:

$$R_{ef,V,k} = n_{ef,ax} \cdot R_{V,k}$$



The n_{ef} value is given in the table below as a function of n (number of pairs).

n_{PAIRS}	2	3	4	5	6	7	8	9	10
$n_{ef,ax}$	1,87	2,70	3,60	4,50	5,40	6,30	7,20	8,10	9,00

INSTALLATION SUGGESTIONS

TIMBER-TO-TIMBER JOINT WITH CROSSED CONNECTORS

TIGHTENING THE JOINT

For correct installation of the joint, we recommend tightening the elements before inserting the connectors.

Insert a partially threaded screw (e.g. HBS680) to bring the elements closer together.

The HBS screw eliminated the initial gap between the elements.
After positioning the VGZ connectors, it can be removed.

INSERTION OF CONNECTORS

To ensure the correct positioning and inclination of the VGZ screws, we recommend using the JIGVGZ45 template.

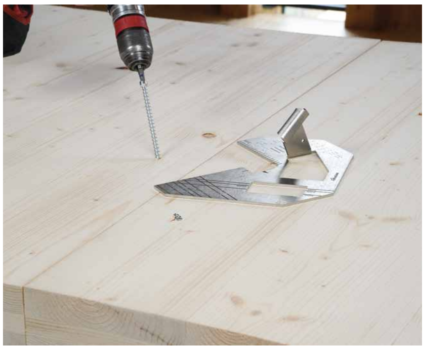
After tightening about one third of the screw, remove the JIGVGZ45 template and continue with the installation.

Repeat the procedure to install the inserted screw from the main beam to the secondary beam.

JOINT BETWEEN CLT PANELS WITH CONNECTORS INCLINED IN BOTH DIRECTIONS (45°-45°)

To ensure the correct positioning and inclination of the VGZ screws, we recommend using the JIGVGZ45 template positioned at 45° to the panel head.

After tightening about one third of the screw, remove the JIGVGZ45 template and continue with the installation.



Repeat the procedure to install the screw in the adjoining panel and continue this alternating sequence according to the distances provided in the design.

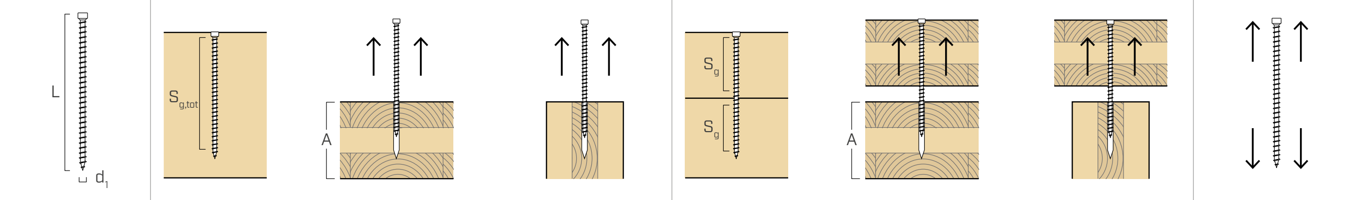
RELATED PRODUCTS

HBS
page 30

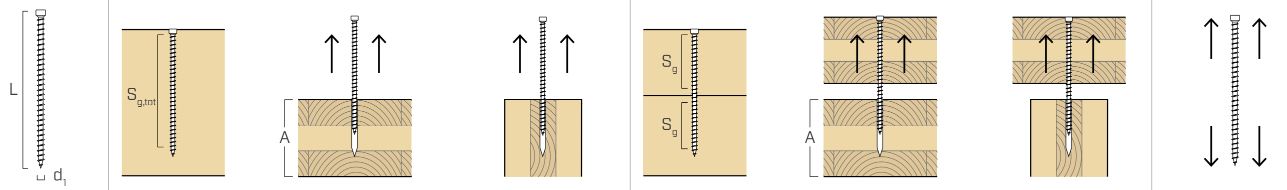
CATCH
page 408

BIT
page 417

JIG VGZ 45°
page 409



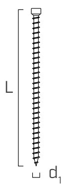
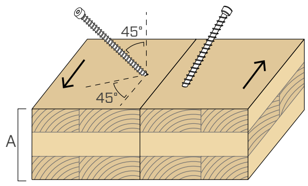
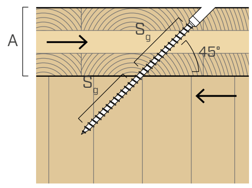
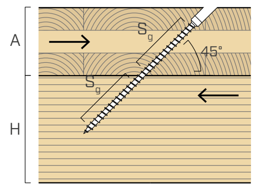
		TENSION								
geometry		total thread withdrawal				partial thread withdrawal				steel tension
		lateral			narrow	lateral			narrow	
d_1 [mm]	L [mm]	$S_{g,tot}$ [mm]	A_{min} [mm]	$R_{ax,90,k}$ [kN]	$R_{ax,0,k}$ [kN]	S_g [mm]	A_{min} [mm]	$R_{ax,90,k}$ [kN]	$R_{ax,0,k}$ [kN]	$R_{tens,k}$ [kN]
	80	70	90	5,73	4,34	-	-	-	-	
	100	90	110	7,37	5,44	35	55	2,87	2,33	
	120	110	130	9,01	6,52	45	65	3,69	2,92	
	140	130	150	10,65	7,58	55	75	4,50	3,49	
	160	150	170	12,29	8,62	65	85	5,32	4,06	
	180	170	190	13,92	9,65	75	95	6,14	4,62	
	200	190	210	15,56	10,67	85	105	6,96	5,17	
	220	210	230	17,20	11,67	95	115	7,78	5,72	
7	240	230	250	18,84	12,67	105	125	8,60	6,25	15,40
	260	250	270	20,48	13,65	115	135	9,42	6,79	
	280	270	290	22,11	14,63	125	145	10,24	7,32	
	300	290	310	23,75	15,61	135	155	11,06	7,84	
	320	310	330	25,39	16,57	145	165	11,88	8,36	
	340	330	350	27,03	17,53	155	175	12,69	8,88	
	360	350	370	28,67	18,48	165	185	13,51	9,39	
	380	370	390	30,30	19,43	175	195	14,33	9,90	
	400	390	410	31,94	20,37	185	205	15,15	10,41	
	160	150	170	15,80	10,54	65	85	6,84	4,97	
	180	170	190	17,90	11,80	75	95	7,90	5,65	
	200	190	210	20,01	13,04	85	105	8,95	6,32	
	220	210	230	22,11	14,27	95	115	10,00	6,99	
	240	230	250	24,22	15,49	105	125	11,06	7,65	
	260	250	270	26,33	16,69	115	135	12,11	8,30	
	280	270	290	28,43	17,89	125	145	13,16	8,95	
	300	290	310	30,54	19,08	135	155	14,22	9,59	
9	320	310	330	32,64	20,26	145	165	15,27	10,22	25,40
	340	330	350	34,75	21,43	155	175	16,32	10,86	
	360	350	370	36,86	22,60	165	185	17,37	11,49	
	380	370	390	38,96	23,76	175	195	18,43	12,11	
	400	390	410	41,07	24,91	185	205	19,48	12,73	
	440	430	450	45,28	27,20	205	225	21,59	13,96	
	480	470	490	49,49	29,47	225	245	23,69	15,18	
	520	510	530	53,70	31,71	245	265	25,80	16,39	
	560	550	570	57,92	33,94	265	285	27,90	17,59	
	600	590	610	62,13	36,16	285	305	30,01	18,78	



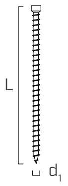
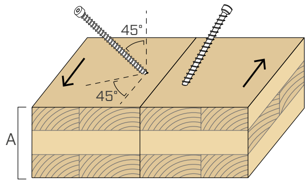
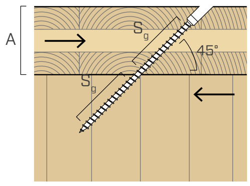
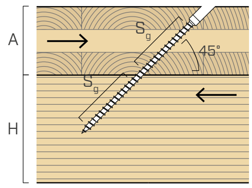
		TENSION								
geometry		total thread withdrawal				partial thread withdrawal				steel tension
		lateral			narrow	lateral			narrow	
d_1 [mm]	L [mm]	$S_{g,tot}$ [mm]	A_{min} [mm]	$R_{ax,90,k}$ [kN]	$R_{ax,0,k}$ [kN]	S_g [mm]	A_{min} [mm]	$R_{ax,90,k}$ [kN]	$R_{ax,0,k}$ [kN]	$R_{tens,k}$ [kN]
	150	140	160	18,02	11,63	60	80	7,72	5,43	
	200	190	210	24,45	15,31	85	105	10,94	7,42	
	250	240	260	30,89	18,89	110	130	14,16	9,36	
	275	265	285	34,11	20,66	123	143	15,77	10,31	
	300	290	310	37,32	22,40	135	155	17,37	11,26	
	325	315	335	40,54	24,13	148	168	18,98	12,19	
	350	340	360	43,76	25,85	160	180	20,59	13,12	
	375	365	385	46,98	27,56	173	193	22,20	14,04	
	400	390	410	50,19	29,25	185	205	23,81	14,95	
	425	415	435	53,41	30,93	198	218	25,42	15,85	
	450	440	460	56,63	32,60	210	230	27,03	16,75	
	475	465	485	59,85	34,27	223	243	28,64	17,65	
11	500	490	510	63,06	35,92	235	255	30,24	18,54	38,00
	525	515	535	66,28	37,56	248	268	31,85	19,43	
	550	540	560	69,50	39,20	260	280	33,46	20,31	
	575	565	585	72,72	40,83	273	293	35,07	21,18	
	600	590	610	75,93	42,45	285	305	36,68	22,05	
	650	640	660	82,37	45,68	310	330	39,90	23,79	
	700	690	710	88,80	48,88	335	355	43,11	25,51	
	750	740	760	95,24	52,05	360	380	46,33	27,22	
	800	790	810	101,67	55,21	385	405	49,55	28,91	
	850	840	860	108,11	58,34	410	430	52,77	30,59	
	900	890	910	114,54	61,46	435	455	55,98	32,27	
	950	940	960	120,98	64,56	460	480	59,20	33,93	
	1000	990	1010	127,41	67,64	485	505	62,42	35,59	

NOTES and GENERAL PRINCIPLES on page 143.

STRUCTURAL VALUES | CLT

			SLIDING									
geometry			CLT - CLT 45° + 45°			CLT - CLT			CLT - timber			
												
d_1	L	S_g	A_{min}	$R_{V,k}$	$R_{tens,45+45,k}$	A	$R_{V,k}$	$R_{tens,45,k}$	A	H_{min}	$R_{V,k}$	$R_{tens,45,k}$
[mm]	[mm]	[mm]	[mm]	[kN]	[kN]	[mm]	[kN]	[kN]	[mm]	[mm]	[kN]	[kN]
	80	25	65	0,86		35	1,22		35	50	1,45	
	100	35	80	1,16		40	1,65		40	55	2,03	
	120	45	95	1,46		45	2,06		45	60	2,61	
	140	55	110	1,75		55	2,47		55	70	3,19	
	160	65	125	2,03		60	2,87		60	75	3,76	
	180	75	135	2,31		70	3,27		70	85	4,34	
	200	85	150	2,59		75	3,66		75	90	4,92	
	220	95	165	2,86		85	4,04		85	100	5,50	
7	240	105	180	3,13	7,70	90	4,42	10,89	90	105	6,08	10,89
	260	115	195	3,39		95	4,80		95	110	6,66	
	280	125	210	3,66		105	5,17		105	120	7,24	
	300	135	220	3,92		110	5,54		110	125	7,82	
	320	145	235	4,18		120	5,91		120	135	8,40	
	340	155	250	4,44		125	6,28		125	140	8,98	
	360	165	265	4,70		130	6,64		130	145	9,56	
	380	175	280	4,95		140	7,00		140	155	10,13	
	400	185	295	5,21		145	7,36		145	160	10,71	
	160	65	125	2,48		60	3,51		60	75	4,84	
	180	75	135	2,82		70	3,99		70	85	5,58	
	200	85	150	3,16		75	4,47		75	90	6,33	
	220	95	165	3,49		85	4,94		85	100	7,07	
	240	105	180	3,82		90	5,41		90	105	7,82	
	260	115	195	4,15		95	5,87		95	110	8,56	
	280	125	210	4,47		105	6,33		105	120	9,31	
	300	135	220	4,79		110	6,78		110	125	10,05	
9	320	145	235	5,11	12,70	120	7,23	17,96	120	135	10,80	17,96
	340	155	250	5,43		125	7,68		125	140	11,54	
	360	165	265	5,74		130	8,12		130	145	12,29	
	380	175	280	6,06		140	8,56		140	155	13,03	
	400	185	295	6,37		145	9,00		145	160	13,77	
	440	205	320	6,98		160	9,87		160	175	15,26	
	480	225	350	7,59		175	10,74		175	190	16,75	
	520	245	380	8,20		190	11,59		190	205	18,24	
	560	265	405	8,80		205	12,44		205	220	19,73	
	600	285	435	9,39		215	13,28		215	230	21,22	

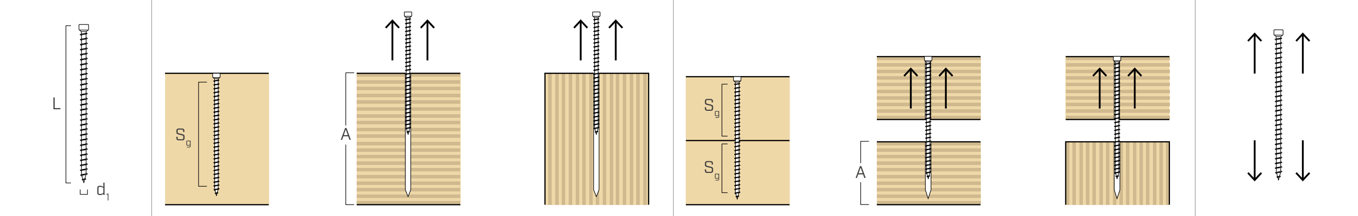
STRUCTURAL VALUES | CLT

			SLIDING									
geometry			CLT - CLT 45° + 45°			CLT - CLT			CLT - timber			
												
d_1 [mm]	L [mm]	S_g [mm]	A_{min} [mm]	$R_{V,k}$ [kN]	$R_{tens,45+45,k}$ [kN]	A [mm]	$R_{V,k}$ [kN]	$R_{tens,45,k}$ [kN]	A [mm]	H_{min} [mm]	$R_{V,k}$ [kN]	$R_{tens,45,k}$ [kN]
	150	60	115	2,71		60	3,84		60	75	5,46	
	200	85	150	3,71		75	5,25		75	90	7,74	
	250	110	185	4,68		95	6,62		95	110	10,01	
	275	123	205	5,16		100	7,29		100	115	11,15	
	300	135	220	5,63		110	7,96		110	125	12,29	
	325	148	240	6,10		120	8,62		120	135	13,42	
	350	160	255	6,56		130	9,28		130	145	14,56	
	375	173	275	7,02		140	9,93		140	155	15,70	
	400	185	295	7,47		145	10,57		145	160	16,84	
	425	198	310	7,93		155	11,21		155	170	17,97	
	450	210	330	8,38		165	11,85		165	180	19,11	
	475	223	345	8,82		175	12,48		175	190	20,25	
11	500	235	365	9,27	19,00	180	13,11	26,87	180	195	21,39	26,87
	525	248	380	9,71		190	13,74		190	205	22,52	
	550	260	400	10,15		200	14,36		200	215	23,66	
	575	273	415	10,59		210	14,98		210	225	24,80	
	600	285	435	11,03		215	15,60		215	230	25,94	
	650	310	470	11,89		235	16,82		235	250	28,21	
	700	335	505	12,75		250	18,04		250	265	30,49	
	750	360	540	13,61		270	19,24		270	285	32,76	
	800	385	575	14,46		290	20,44		290	305	35,04	
	850	410	610	15,30		305	21,63		305	320	37,31	
	900	435	645	16,13		325	22,82		325	340	39,59	
	950	460	680	16,97		340	23,99		340	355	41,86	
	1000	485	715	17,79		360	25,16		360	375	44,14	

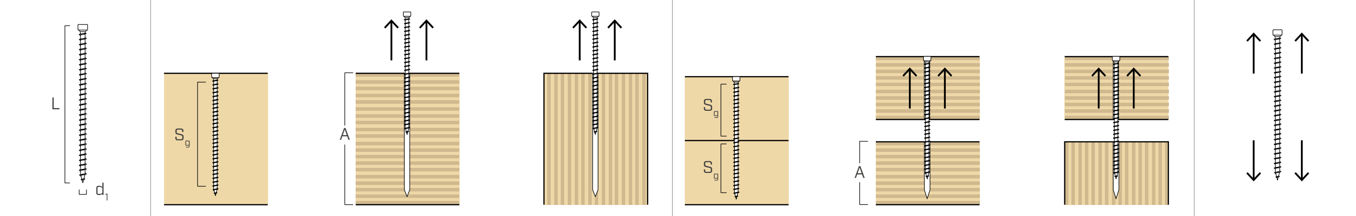
NOTES | CLT

- The characteristic values are according to the national specifications ÖNORM EN 1995 - Annex K.
- For the calculation process, a mass density of ρ_k = 350 kg/m^3 has been considered for CLT elements and a mass density of ρ_k = 385 kg/m^3 has been considered for timber elements.
- The axial thread withdrawal resistance in the narrow face is valid for minimum CLT thickness $t_{CLT,min} = 10 \cdot d_1$ and minimum screw pull-through depth $t_{pen} = 10 \cdot d_1$.
- The characteristic sliding strengths of the connectors inserted in the lateral face of the CLT panel were evaluated considering an angle ε of 45° between the grains and the connector, since it was not possible to define the thickness and orientation of the individual layers in advance.
- The characteristic sliding strengths of the connectors inserted with double inclination (45°-45°) were evaluated considering an ε angle of 60° between the grains and the connector; in fact, the geometry of the joint requires that the connectors have to be inserted at an angle of 45° with respect to the face of the CLT panel and at an angle of 45° with respect to the shear plane between the two panels.
  The use of the JIG VGZ 45 template is recommended for professional installation of the connectors in this application.
- Connectors instability must be verified separately.

GENERAL PRINCIPLES on page 143.

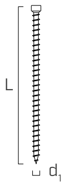


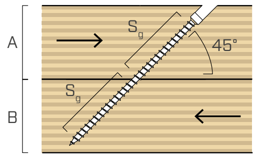
geometry		TENSION								
		total thread withdrawal				partial thread withdrawal				steel tension
		wide			edge	wide			edge	
d_1 [mm]	L [mm]	$S_{g,tot}$ [mm]	A_{min} [mm]	$R_{ax,90,k}$ [kN]	$R_{ax,0,k}$ [kN]	S_g [mm]	A_{min} [mm]	$R_{ax,90,k}$ [kN]	$R_{ax,0,k}$ [kN]	$R_{tens,k}$ [kN]
7	80	70	90	7,11	4,74	-	-	-	-	15,40
	100	90	110	9,15	5,44	35	55	3,56	2,37	
	120	110	130	11,18	6,52	45	65	4,57	3,05	
	140	130	150	13,21	7,58	55	75	5,59	3,73	
	160	150	170	15,24	8,62	65	85	6,61	4,40	
	180	170	190	17,28	9,65	75	95	7,62	5,08	
	200	190	210	19,31	10,67	85	105	8,64	5,76	
	220	210	230	21,34	11,67	95	115	9,65	6,44	
	240	230	250	23,37	12,67	105	125	10,67	7,11	
	260	250	270	25,41	13,65	115	135	11,69	7,79	
	280	270	290	27,44	14,63	125	145	12,70	8,47	
	300	290	310	29,47	15,61	135	155	13,72	9,15	
	320	310	330	31,50	16,57	145	165	14,74	9,82	
	340	330	350	33,54	17,53	155	175	15,75	10,50	
	360	350	370	35,57	18,48	165	185	16,77	11,18	
	380	370	390	37,60	19,43	175	195	17,78	11,86	
	400	390	410	39,63	20,37	185	205	18,80	12,53	
9	160	150	170	19,60	10,54	65	85	8,49	5,66	25,40
	180	170	190	22,21	11,80	75	95	9,80	6,53	
	200	190	210	24,83	13,04	85	105	11,11	7,40	
	220	210	230	27,44	14,27	95	115	12,41	8,28	
	240	230	250	30,05	15,49	105	125	13,72	9,15	
	260	250	270	32,67	16,69	115	135	15,03	10,02	
	280	270	290	35,28	17,89	125	145	16,33	10,89	
	300	290	310	37,89	19,08	135	155	17,64	11,76	
	320	310	330	40,51	20,26	145	165	18,95	12,63	
	340	330	350	43,12	21,43	155	175	20,25	13,50	
	360	350	370	45,73	22,60	165	185	21,56	14,37	
	380	370	390	48,35	23,76	175	195	22,87	15,24	
	400	390	410	50,96	24,91	185	205	24,17	16,12	
	440	430	450	56,18	27,20	205	225	26,79	17,86	
	480	470	490	61,41	29,47	225	245	29,40	19,60	
	520	510	530	66,64	31,71	245	265	32,01	21,34	
	560	550	570	71,86	33,94	265	285	34,63	23,08	
	600	590	610	77,09	36,16	285	305	37,24	24,83	

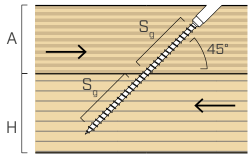


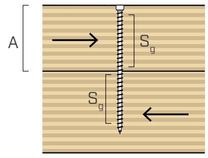
geometry		TENSION								
		total thread withdrawal				partial thread withdrawal				steel tension
		wide			edge	wide			edge	
d_1 [mm]	L [mm]	$S_{g,tot}$ [mm]	A_{min} [mm]	$R_{ax,90,k}$ [kN]	$R_{ax,0,k}$ [kN]	S_g [mm]	A_{min} [mm]	$R_{ax,90,k}$ [kN]	$R_{ax,0,k}$ [kN]	$R_{tens,k}$ [kN]
	150	140	160	22,36	11,63	60	80	9,58	6,39	
	200	190	210	30,34	15,31	85	105	13,57	9,05	
	250	240	260	38,33	18,89	110	130	17,57	11,71	
	275	265	285	42,32	20,66	123	143	19,56	13,04	
	300	290	310	46,31	22,40	135	155	21,56	14,37	
	325	315	335	50,31	24,13	148	168	23,56	15,70	
	350	340	360	54,30	25,85	160	180	25,55	17,03	
	375	365	385	58,29	27,56	173	193	27,55	18,37	
	400	390	410	62,28	29,25	185	205	29,54	19,70	
	425	415	435	66,27	30,93	198	218	31,54	21,03	
	450	440	460	70,27	32,60	210	230	33,54	22,36	
	475	465	485	74,26	34,27	223	243	35,53	23,69	
11	500	490	510	78,25	35,92	235	255	37,53	25,02	38,00
	525	515	535	82,24	37,56	248	268	39,53	26,35	
	550	540	560	86,24	39,20	260	280	41,52	27,68	
	575	565	585	90,23	40,83	273	293	43,52	29,01	
	600	590	610	94,22	42,45	285	305	45,51	30,34	
	650	640	660	102,21	45,68	310	330	49,51	33,00	
	700	690	710	110,19	48,88	335	355	53,50	35,67	
	750	740	760	118,18	52,05	360	380	57,49	38,33	
	800	790	810	126,16	55,21	385	405	61,48	40,99	
	850	840	860	134,15	58,34	410	430	65,48	43,65	
	900	890	910	142,13	61,46	435	455	69,47	46,31	
	950	940	960	150,12	64,56	460	480	73,46	48,97	
	1000	990	1010	158,10	67,64	485	505	77,45	51,64	

NOTES and GENERAL PRINCIPLES on page 143.

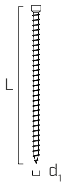
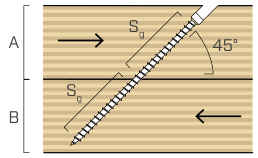
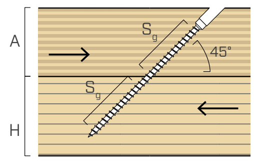
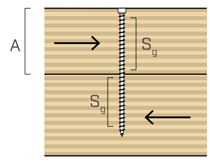








			SLIDING								SHEAR	
geometry			LVL-LVL				LVL-timber				LVL-LVL wide	
d_1	L	S_g	A	B_{min}	$R_{V,k}$	$R_{tens,45,k}$	A	H_{min}	$R_{V,k}$	$R_{tens,45,k}$	A	$R_{V,90,k}$
[mm]	[mm]	[mm]	[mm]	[mm]	[kN]	[kN]	[mm]	[mm]	[kN]	[kN]	[mm]	[kN]
7	100	35	40	55	2,01	10,89	40	45	2,01	10,89	50	3,29
	120	45	45	60	2,59		45	50	2,59		60	3,55
	140	55	55	70	3,16		55	60	3,16		70	3,80
	160	65	60	75	3,74		60	65	3,74		80	4,05
	180	75	70	85	4,31		70	75	4,31		90	4,31
	200	85	75	90	4,89		75	80	4,89		100	4,56
	220	95	85	100	5,46		85	90	5,46		110	4,81
	240	105	90	105	6,04		90	95	6,04		120	4,81
	260	115	95	110	6,61		95	100	6,61		130	4,81
	280	125	105	120	7,19		105	110	7,19		140	4,81
	300	135	110	125	7,76		110	115	7,76		150	4,81
	320	145	120	135	8,34		120	125	8,34		160	4,81
	340	155	125	140	8,91		125	130	8,91		170	4,81
	360	165	130	145	9,49		130	135	9,49		180	4,81
	380	175	140	155	10,06		140	145	10,06		190	4,81
	400	185	145	160	10,64		145	150	10,64		200	4,81
9	160	65	60	75	4,80	17,96	60	65	4,80	17,96	80	5,75
	180	75	70	85	5,54		70	75	5,54		90	6,08
	200	85	75	90	6,28		75	80	6,28		100	6,41
	220	95	85	100	7,02		85	90	7,02		110	6,73
	240	105	90	105	7,76		90	95	7,76		120	7,06
	260	115	95	110	8,50		95	100	8,50		130	7,26
	280	125	105	120	9,24		105	110	9,24		140	7,26
	300	135	110	125	9,98		110	115	9,98		150	7,26
	320	145	120	135	10,72		120	125	10,72		160	7,26
	340	155	125	140	11,46		125	130	11,46		170	7,26
	360	165	130	145	12,20		130	135	12,20		180	7,26
	380	175	140	155	12,93		140	145	12,93		190	7,26
	400	185	145	160	13,67		145	150	13,67		200	7,26
	440	205	160	175	15,15		160	165	15,15		220	7,26
	480	225	175	190	16,63		175	180	16,63		240	7,26
	520	245	190	205	18,11		190	195	18,11		260	7,26
	560	265	205	220	19,59		205	210	19,59		280	7,26
	600	285	215	230	21,07		215	220	21,07		300	7,26

			SLIDING								SHEAR	
geometry			LVL-LVL				LVL-timber				LVL-LVL wide	
												
d_1 [mm]	L [mm]	S_g [mm]	A [mm]	B_{min} [mm]	$R_{V,k}$ [kN]	$R_{tens,45,k}$ [kN]	A [mm]	H_{min} [mm]	$R_{V,k}$ [kN]	$R_{tens,45,k}$ [kN]	A [mm]	$R_{V,90,k}$ [kN]
	150	60	60	75	5,42		60	65	5,42		75	7,46
	200	85	75	90	7,68		75	80	7,68		100	8,45
	250	110	95	110	9,94		95	100	9,94		125	9,45
	275	123	100	115	11,07		100	105	11,07		138	9,95
	300	135	110	125	12,20		110	115	12,20		150	10,12
	325	148	120	135	13,33		120	125	13,33		163	10,12
	350	160	130	145	14,45		130	135	14,45		175	10,12
	375	173	140	155	15,58		140	145	15,58		188	10,12
	400	185	145	160	16,71		145	150	16,71		200	10,12
	425	198	155	170	17,84		155	160	17,84		213	10,12
	450	210	165	180	18,97		165	170	18,97		225	10,12
	475	223	175	190	20,10		175	180	20,10		238	10,12
11	500	235	180	195	21,23	26,87	180	185	21,23	26,87	250	10,12
	525	248	190	205	22,36		190	195	22,36		263	10,12
	550	260	200	215	23,49		200	205	23,49		275	10,12
	575	273	210	225	24,62		210	215	24,62		288	10,12
	600	285	215	230	25,75		215	220	25,75		300	10,12
	650	310	235	250	28,01		235	240	28,01		325	10,12
	700	335	250	265	30,26		250	255	30,26		350	10,12
	750	360	270	285	32,52		270	275	32,52		375	10,12
	800	385	290	305	34,78		290	295	34,78		400	10,12
	850	410	305	320	37,04		305	310	37,04		425	10,12
	900	435	325	340	39,30		325	330	39,30		450	10,12
	950	460	340	355	41,56		340	345	41,56		475	10,12
	1000	485	360	375	43,81		360	365	43,81		500	10,12

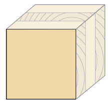
NOTES

- For the calculation process, a mass density of ρ_k = 480 kg/m^3 has been considered for the softwood LVL elements and a mass density of ρ_k = 385 kg/m^3 has been considered for timber elements.
- The axial "wide"thread withdrawal resistance was evaluated considering an angle of 90° between the fibers and the connector and is valid in application with LVLs in both parallel and cross grain veneer beams.
- The axial "edge" thread withdrawal resistance was evaluated considering an angle of 90° between the fibers and the connector and is valid in application with parallel veneer LVLs.
- Minimum height LVL $h_{LVL,min}$= 100 mm for VGZ connectors Ø7 and $h_{LVL,min}$ = 120 mm for VGZ connectors Ø9.
- The characteristic sliding strengths were evaluated by considering, for individual timber elements, a 45° angle between the connector and the grain and a 45° angle between the connector and the side face of the LVL element.
- The characteristic shear strengths were evaluated by considering, for individual timber elements, a 90° angle between the connector and the grain, a 90° angle between the connector and the side face of the LVL element and a 0° angle between the force and the grain.
- Connectors instability must be verified separately.

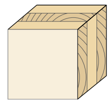
GENERAL PRINCIPLES on page 143.

MINIMUM DISTANCES FOR SHEAR AND AXIAL LOADS | CLT

screws inserted **WITHOUT pre-drilled hole**



lateral face

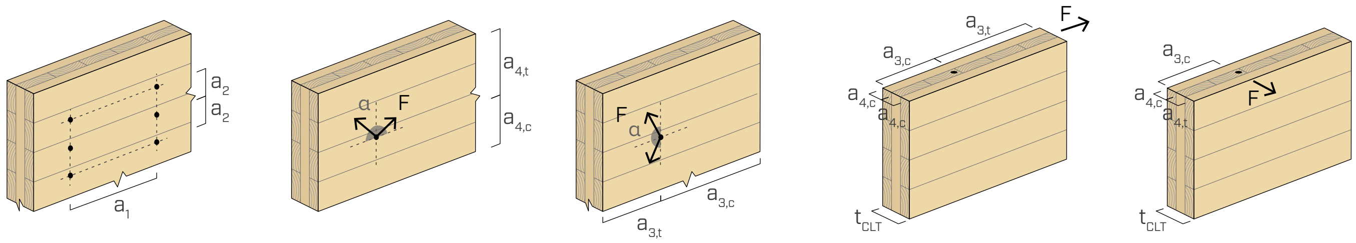


narrow face

d_1	[mm]		7	9	11
a_1	[mm]	4·d	28	36	44
a_2	[mm]	2,5·d	18	23	28
$a_{3,t}$	[mm]	6·d	42	54	66
$a_{3,c}$	[mm]	6·d	42	54	66
$a_{4,t}$	[mm]	6·d	42	54	66
$a_{4,c}$	[mm]	2,5·d	18	23	28

d_1	[mm]		7	9	11
a_1	[mm]	10·d	70	90	110
a_2	[mm]	4·d	28	36	44
$a_{3,t}$	[mm]	12·d	84	108	132
$a_{3,c}$	[mm]	7·d	49	63	77
$a_{4,t}$	[mm]	6·d	42	54	66
$a_{4,c}$	[mm]	3·d	21	27	33

d = d_1 = nominal screw diameter

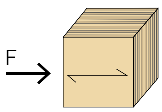


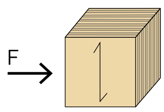
NOTES

- The minimum distances are compliant with ETA-11/0030 and are to be considered valid unless otherwise specified in the technical documents for the CLT panels.
- Minimum distances are valid for minimum CLT thickness $t_{CLT,min}$ =10·d_1.
- The minimum distances referred to "narrow face" are valid for minimum screw pull-through depth t_{pen} = 10·d_1.

MINIMUM DISTANCES FOR SHEAR LOADS | LVL

screws inserted **WITHOUT pre-drilled hole**

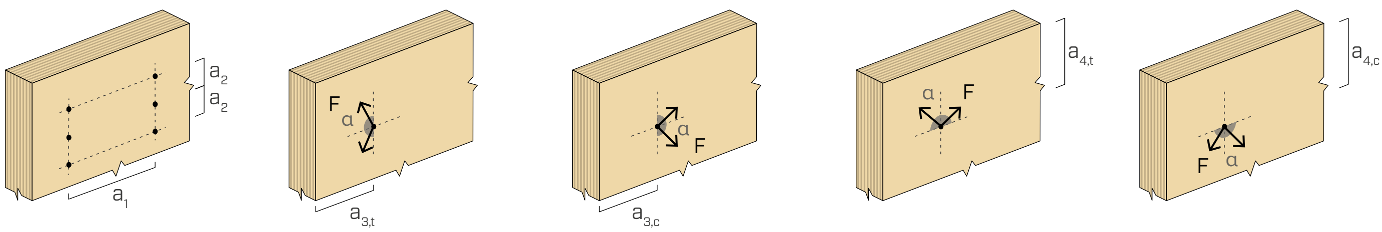




d_1	[mm]		7	9	11
a_1	[mm]	15·d	105	135	165
a_2	[mm]	7·d	49	63	77
$a_{3,t}$	[mm]	20·d	140	180	220
$a_{3,c}$	[mm]	15·d	105	135	165
$a_{4,t}$	[mm]	7·d	49	63	77
$a_{4,c}$	[mm]	7·d	49	63	77

d_1	[mm]		7	9	11
a_1	[mm]	7·d	49	63	77
a_2	[mm]	7·d	49	63	77
$a_{3,t}$	[mm]	15·d	105	135	165
$a_{3,c}$	[mm]	15·d	105	135	165
$a_{4,t}$	[mm]	12·d	84	108	132
$a_{4,c}$	[mm]	7·d	49	63	77

α = load-to-grain angle
d = d_1 = nominal screw diameter

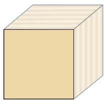


NOTES

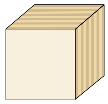
- Minimum distances are obtained from experimental tests carried out at Eurofins Expert Services Oy, Espoo, Finland (Report EUFI29-19000819-T1/T2).

MINIMUM DISTANCES FOR AXIAL STRESSES | LVL

screws inserted **WITHOUT pre-drilled hole**



wide face



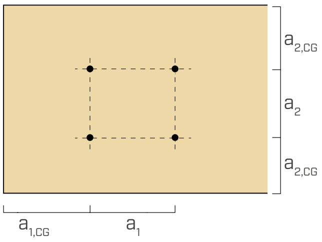
edge face

d_1	[mm]		7	9	11
a_1	[mm]	5·d	35	45	55
a_2	[mm]	5·d	35	45	55
$a_{1,CG}$	[mm]	10·d	70	90	110
$a_{2,CG}$	[mm]	4·d	28	36	44

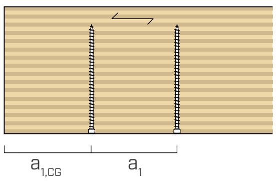
d_1	[mm]		7	9	11
a_1	[mm]	10·d	70	90	110
a_2	[mm]	5·d	35	45	55
$a_{1,CG}$	[mm]	12·d	84	108	132
$a_{2,CG}$	[mm]	3·d	21	27	33

d = d_1 = nominal screw diameter

SCREWS INSERTED WITH α = 90° ANGLE WITH RESPECT TO THE GRAIN (wide face)



plan

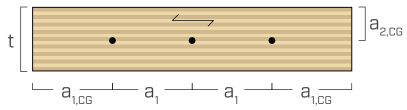


front

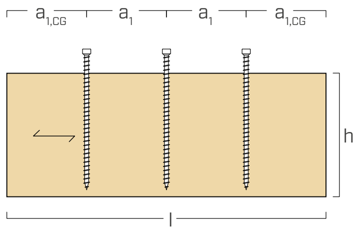
NOTES

- The minimum distances for Ø7 and Ø9 screws with 3 THORNS bit are compliant with ETA-11/0030 and are to be considered valid unless otherwise specified in the technical documents for the LVL panels.
  For Ø11 or self-drilling bit screws, the minimum distances are obtained from experimental tests carried out at Eurofins Expert Services Oy, Espoo, Finland (Report EU-FI29-19000819-T1/T2).
- The minimum distances referred to edge face for screws d = 7 mm are valid for minimum thickness LVL $t_{LVL,min}$ = 45 mm and minimum height LVL $h_{LVL,min}$ = 100 mm.
  The minimum distances referred to edge face for screws d = 9 mm are valid for minimum thickness LVL $t_{LVL,min}$ = 57 mm and minimum height LVL $h_{LVL,min}$ = 120 mm.

SCREWS INSERTED WITH α = 90° ANGLE WITH RESPECT TO THE GRAIN (edge face)



plan



front

STRUCTURAL VALUES

GENERAL PRINCIPLES

- Characteristic values comply with the EN 1995:2014 standard in accordance with ETA-11/0030.
- The tensile design strength of the connector is the lower between the timber-side design strength ($R_{ax,d}$) and the steel-side design strength ($R_{tens,d}$).

$$R_{ax,d} = min \begin{cases} \dfrac{R_{ax,k} \cdot k_{mod}}{\gamma_M} \\ \dfrac{R_{tens,k}}{\gamma_{M2}} \end{cases}$$

- The compression design strength of the connector is the lower between the timber-side design strength ($R_{ax,d}$) and the instability design strength ($R_{ki,d}$).

$$R_{ax,d} = min \begin{cases} \dfrac{R_{ax,k} \cdot k_{mod}}{\gamma_M} \\ \dfrac{R_{ki,k}}{\gamma_{M1}} \end{cases}$$

- The design sliding strength of the joint is either the timber-side design strength ($R_{V,d}$) and the design strength on the steel side projected at 45°. ($R_{tens,45,d}$), whichever is lower:

$$R_{V,d} = min \begin{cases} \dfrac{R_{V,k} \cdot k_{mod}}{\gamma_M} \\ \dfrac{R_{tens,45,k}}{\gamma_{M2}} \end{cases}$$

- The design shear strength of the connector is obtained from the characteristic value as follows:

$$R_{V,d} = \frac{R_{V,k} \cdot k_{mod}}{\gamma_M}$$

- The coefficients γ_M and k_{mod} should be taken according to the current regulations used for the calculation.
- For the mechanical resistance values and the geometry of the screws, reference was made to ETA-11/0030.
- Dimensioning and verification of the timber elements must be carried out separately.
- The screws must be positioned in accordance with the minimum distances.
- The characteristic thread withdrawal strengths were evaluated considering a penetration length of $S_{g,tot}$ or S_g, as shown in the table.
  For intermediate values of S_g it is possible to linearly interpolate. A minimum penetration length of 4-d_1 is considered.
- The shear srength and sliding values were evaluated considering the centre of gravity of the connector placed in correspondence with the shear plane.
- The characteristic shear resistances are calculated for screws inserted without pre-drilling hole. In the case of screws inserted with pre-drilling hole, greater resistance values can be obtained.
- For different calculation configurations, the MyProject software is available (www.rothoblaas.com).